

EXISTING CONDITIONS AND COORDINATION.

- PROJECT PLANS HAVE BEEN DEVELOPED FROM A VISUAL EXAMINATION (WHERE APPLICABLE) OF THE EXISTING BUILDING AND/OR PROJECT PLANS PROVIDED BY THE ARCHITECT OR PROJECT MANAGER. ACTUAL CONDITIONS MAY VARY; CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND DETAILS RELATED TO EXISTING CONSTRUCTION AND CONDITIONS AND MAKE MINOR ADJUSTMENTS AS REQUIRED. REPORT SIGNIFICANT DIFFERENCES TO ARCHITECT/ENGINEER.
- SEE ARCHITECTURAL PLANS FOR ALL DIMENSIONS NOT SHOWN.
- THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL PROJECT DRAWINGS AND SPECIFICATIONS INCLUDING THESE STRUCTURAL DRAWINGS.
- APPLICABLE SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW PRIOR TO PRODUCTION.
- FINISH FLOOR ELEVATION IS TO TOP OF PLYWOOD DECKING OR TOP OF SLAB AS APPLICABLE UNO.
- THE CONTRACTOR SHALL NOTIFY THE ARCHITECT / SEOR OF ANY POTENTIAL OR DISCOVERED CONFLICTS, OMISSIONS OR DISCREPANCIES IN THE CONTRACT DRAWINGS OR OF RELATED TO THE STRUCTURAL DESIGN OF THE BUILDING PRIOR TO PROCEEDING.
- THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR PROVIDING DESIGN OR COORDINATING OTHER ENGINEERING DISCIPLINES, ARCHITECTURAL OR DETAILING, OR ANY OTHER NON-STRUCTURAL CODES OR STANDARDS. EVERY EFFORT HAS BEEN MADE TO ACCOMMODATE AND COORDINATE THESE ITEMS, BUT FINAL COORDINATION AND COMPLIANCE IS THE RESPONSIBILITY OF THE ARCHITECT, PROJECT MANAGER, OR OWNER AS IS APPLICABLE FOR THE PROJECT.

STATEMENT OF SPECIAL INSPECTIONS

- SPECIAL INSPECTIONS ARE REQUIRED IN ACCORDANCE WITH SECTION 17 OF THE REFERENCED EDITION OF THE IRC, THE MATERIAL, SYSTEMS, COMPONENTS, AND WORK REQUIRED TO HAVE SPECIAL INSPECTIONS OR TESTS ARE INDICATED IN THE SCHEDULE OF SPECIAL INSPECTIONS. THE TYPE OF EACH SPECIAL INSPECTION OR TEST IS NOTED IN THE SPECIAL INSPECTION SCHEDULE. THE FREQUENCY OF THE SPECIAL INSPECTION (PERIODIC / CONTINUOUS) IS NOTED WITH THE SPECIAL INSPECTION SCHEDULE.
- THE SEOR IS NOT RESPONSIBLE FOR PERFORMING THE SPECIAL INSPECTIONS ON THIS PROJECT. A QUALIFIED INSPECTOR SHALL BE REQUIRED IN ACCORDANCE WITH IBC 1704.2.1.
 - ANY ADDITIONAL STRUCTURAL OBSERVATIONS IN ACCORDANCE WITH IBC 1704.6 ARE NOTED ON THESE DRAWINGS.

GEOTECHNICAL INFORMATION

- A GEOTECHNICAL SUBSURFACE INVESTIGATION HAS NOT BEEN PROVIDED FOR THIS PROJECT. FOUNDATION DESIGN IS BASED ON SUSPECTED SUBSURFACE CONDITIONS TYPICAL FOR THE AREA. FOOTINGS ARE SIZED FOR A SOIL BEARING VALUE AS NOTED BELOW.
- THE CONTRACTOR SHALL VERIFY THE CAPABILITY OF THE SOIL STRATA TO SUPPORT FOUNDATIONS PRIOR TO CASTING THE FOUNDATION.
- THE FOUNDATION SHALL EXTEND TO A MINIMUM OF THE FROST PENETRATION DEPTH, TO A DEPTH WHERE SOIL MOISTURE CONTENT DOES NOT FLUCTUATE, A MINIMUM DEPTH OF 24" INTO ORIGINAL SOIL OR A MINIMUM DEPTH TO ACHIEVE THE BELOW NOTED BEARING CAPACITY (WHICHEVER IS GREATER).
- NOTIFY THE ENGINEER SHOULD ANY UNUSUAL SOIL CONDITIONS BE ENCOUNTERED.

FOUNDATIONS

- THE "CONTROLLED AREA" SHALL EXTEND BENEATH AND 5 FEET BEYOND THE BUILDING AREA, THE "CONTROLLED AREA" SHALL BE COMPLETELY STRIPPED AND ALL SURFACE VEGETATION, ORGANIC FILL OR TOPSOIL, DEBRIS AND ANY OTHER.
- THE SUBGRADE ELEVATION SHALL BE ESTABLISHED BY CONSTRUCTION OF AN ENGINEERED FILL USING SUITABLE FILL EARTH AND PLACED IN LIFTS NOT TO EXCEED 8". THE SUBGRADE SHALL BE DENSIFIED TO 95% (MIN) STANDARD DENSITY (ASTM D698), VERIFYING IN PLACE DENSITY TESTS ARE REQUIRED.
- THE CONTRACTOR SHALL DEWATER THE AREA OF THE BUILDING FOOTPRINT AS REQUIRED TO PREVENT PONDING OF WATER IN THE FOOTING TRENCHES AND FLARED CORNERS EXCAVATION AND PRIOR TO CASTING FOOTINGS.
- ASSUMED BEARING CAPACITY AS LISTED BELOW SHALL BE VERIFIED PRIOR TO CASTING FOOTINGS.
- FOUNDATION DESIGN PARAMETERS:
 - ALLOWABLE BEARING CAPACITY CONTINUOUS SPREAD FOOTING 2000 PSF
 - ALLOWABLE BEARING CAPACITY ISOLATED SPREAD FOOTING 2000 PSF
 - MINIMUM PERIMETER FOOTING BEARING DEPTH BELOW FINISH GRADE 24"
 - MINIMUM INTERIOR FOOTING BEARING DEPTH BELOW FINISH FLOOR ELEVATION 12"

CONCRETE

- CONCRETE SHALL CONFORM TO THE BUILDING CODE REQUIREMENT FOR REINFORCED CONCRETE (ACI 318).
- CONCRETE SHALL HAVE THE FOLLOWING COMPRESSIVE STRENGTH (FC) AT 28 DAYS BASED UPON ITS USE:
 - FOOTINGS 3000 PSI (MIN.)
 - SLAB ON GRADE 3000 PSI (MIN.)
- REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60.
- WELDED WIRE FABRIC (W.W.F.) SHALL CONFORM TO ASTM A1064.
- SLABS ON GRADE SHALL BE REINFORCED AS INDICATED ON PLANS W.W.F. PLACED AT 1/3 SLAB THICKNESS FROM TOP.
- W.W.F. SHALL BE SUPPORTED ON PLASTIC CHAIRS OR CEMENTitious BLOCKS AT THE CORRECT HEIGHT AS NOTED ABOVE AND AS SHOWN IN DETAILS.
- CAST IN PLACE ANCHOR RODS SHALL CONFORM TO ASTM F 1554 GR. 36.
- MINIMUM CONCRETE COVER (UNLESS OTHERWISE NOTED ON DRAWINGS) FOR REINFORCING SHALL BE:
 - CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3 IN.
 - EXPOSED TO EARTH OR WEATHER 2 IN.
 - BEAMS AND COLUMNS 1 1/2 IN.
- LAP ALL CONTINUOUS REINFORCEMENT WITH A CLASS B LAP SPlice AS SPECIFIED IN LAP SPlice SCHEDULE.
- AT EXTERIOR BUILDING CORNERS FOOTINGS, PROVIDE 3/4" X 3/4" CORNER BARS, SAME SIZE AND NUMBER AS DETAILED HORIZONTAL BARS.
- DOWEL ALL FOOTINGS WHERE THEY ABUT WITH SAME REINFORCEMENT AS DETAILED HORIZONTALLY AND WITH 24" MINIMUM LAP.
- CAST IN PLACE CONCRETE WALLS ARE UNSTABLE AND REQUIRE TEMPORARY CONSTRUCTION BRACING UNTIL INSTALLATION OF PERMANENT CONNECTION. TEMPORARY CONSTRUCTION BRACING IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR
- SUBMITTALS
 - CONCRETE MIX DESIGNS
 - SUBMITTALS SHALL BE IN ACCORDANCE WITH ACI 301 AND ACI 318 (LATEST EDITIONS) PRIOR TO COMMENCEMENT OF CONCRETE WORK.
 - SUBMITTAL SHALL BE REVIEWED AND APPROVED BY THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO SCHEDULING CONCRETE DELIVERY TO JOB SITE.
- REINFORCEMENT SUBMITTALS
 - SUBMITTALS SHALL BE IN ACCORDANCE WITH ACI 315 (LATEST EDITION) AND SHOW, AT MINIMUM, ALL SIZES, DIMENSIONS, LOCATIONS OF ALL REINFORCEMENT AND EMBEDMENTS.
 - SUBMITTAL SHALL BE REVIEWED AND APPROVED BY THE ENGINEER OF RECORD PRIOR TO FABRICATING REINFORCEMENT.

MASONRY

- CONCRETE MASONRY UNITS SHALL BE HOLLOW LOAD BEARING CONFORMING TO ASTM C 90 ALL LOCATIONS.
- MORTAR SHALL BE PROPORTIONED IN ACCORDANCE WITH ASTM C 270.
- GROUT SHALL BE PROPORTIONED IN ACCORDANCE WITH ASTM C 270.
- TEST BLOCKS FOR BELOW GROUND LEVEL AND OTHER TEST BLOCKS FOR ABOVE GROUND CONFORMING TO ASTM C-270.
- MINIMUM INDIVIDUAL NET AREA COMPRESSIVE STRENGTH OF SINGLE CMU 2000 PSI
- MINIMUM DESIGN STRENGTH OF MASONRY (FM) 2000 PSI
- GROUT COMPRESSIVE STRENGTH 3000 PSI
- HORIZONTAL JOINT REINFORCING SHALL BE LADDER TYPE FABRICATED WITH A SINGLE PAIR OF 3 GAGE SIDE RODS AND 3 GAGE CROSS RODS SPACED NOT MORE THAN 16" O.C. REINFORCEMENT SHALL BE FOR TOTAL WIDTH OF SINGLE AND MULTIPLE WEB UNIT WALLS.
- FILLED CELLS INDICATED ON PLAN SHALL BE FILLED WITH GROUT IN LIFTS OF 48" (MAX), TERMINATE LIFT 1'-1/2" BELOW BED JOINT TO CREATE SHEAR KEY TO NEXT LIFT.
- STARTER DOWELS AND EACH ADDITIONAL VERTICAL BAR SHALL BE TIED IN ACCORDANCE WITH TMS SPECIFICATIONS AND LAPPED PER CMU LAP SCHEDULE.
- "WET SETTING" DOWELS SHALL NOT BE ALLOWED.
- MASONRY WALLS ARE UNSTABLE AND REQUIRE TEMPORARY CONSTRUCTION BRACING UNTIL INSTALLATION OF PERMANENT CONNECTION. TEMPORARY CONSTRUCTION BRACING IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- MASONRY CONTROL JOINTS (MCJ) SHALL BE REQUIRED WITH SPACING SHOWN ON ARCHITECTURAL PLAN. MAXIMUM SPACING OF 20' OR 1/3 WALL HEIGHT ALONG WALL LENGTH AND 124" MAX FROM WALL CORNERS. CONSTRUCT AS SHOWN ON MASONRY CONTROL JOINT DETAIL ON STRUCTURAL DRAWINGS.

STRUCTURAL STEEL

- SECTION SHAPES SHALL CONFORM TO ASTM A992.
- RECTANGULAR HSS SHALL CONFORM TO ASTM A500 GR. C.
- ROUND HSS SHALL CONFORM TO ASTM A500 GR. C.
- STRUCTURAL AND MISCELLANEOUS STEEL ITEMS SHALL CONFORM TO ASTM A36.
- BOLTS SHALL BE ASTM A490X.
- NUTS SHALL BE ASTM A503.
- WASHERS SHALL BE ASTM F436.
- STEEL HEADED STUDS SHALL BE ASTM A108.
- DETAIL FABRICATION AND ERECTION OF ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH LATEST AISC STANDARDS AND SPECIFICATIONS.
- ALL WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1 (LATEST EDITION)
- ELECTRODES SHALL BE E70XX.
- UNLESS OTHERWISE NOTED OR DETAIL ED, ALL SHEAR CONNECTIONS SHALL BE DESIGNED USING THE APPROPRIATE DATA FROM PART 16 "DESIGN OF SIMPLE SHEAR CONNECTIONS" FROM THE AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION. DESIGN AND REACTION IS 60% OF TOTAL ALLOWABLE LOAD (60% X WOT) FROM THE ALLOWABLE LOAD OF BEAM TABLE FROM PART 3 - DESIGN OF CONNECTING ELEMENTS" OF THE AISC MANUAL OF STEEL CONSTRUCTION, LATEST EDITION.

STEEL JOISTS AND JOIST ORDERS

- STEEL JOIST MANUFACTURER SHALL BE A CURRENT MEMBER OF THE STEEL JOIST INSTITUTE (SJI)
- STEEL JOISTS AND JOIST ORDERS SHALL CONFORM TO THE SPECIFICATIONS AND REQUIREMENTS OF THE LATEST EDITION OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOIST (K AND LH SERIES) AND JOIST ORDERS.
- JOIST BRACING SHALL BE FURNISHED AND INSTALLED TO MEET THE DESIGN AND BRACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS. UPLIFT BRACING AS REQUIRED SHALL BE PROVIDED BY THE JOIST MANUFACTURER AND CLEARLY SHOWN ON THE ERECTION DRAWINGS. BRACING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS.
- PRIOR TO FABRICATION, SUBMIT SHOP AND LAYOUT DRAWINGS IN SUFFICIENT DETAIL TO DEFINE THE LOCATION OF THE JOISTS, BRIDGING, BRACES, BRIDGES, HEADERS AND OTHER ACCESSORIES FOR REVIEW BY THE ENGINEER OF RECORD.

DELEGATED STEEL SHOP AND HANDRAIL DESIGN

- CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING THE DESIGN OF THE PREFABRICATED STAIRS, LANDINGS, TREADS, AND HANDRAILS WITH AN APPROVED STEEL FABRICATOR. THE FABRICATOR SHALL DESIGN AND DETAIL ALL RELEVANT MEMBERS AND CONNECTIONS.
- THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS AND DRAWING CALCULATIONS PRIOR TO FABRICATION. THE CALCULATIONS SHALL BEAR THE SEAL OF REGISTERED PROFESSIONAL ENGINEER OF THE STATE OF PROJECT.

DEFERRED DESIGN SUBMITTALS

- THE FOLLOWING ITEMS ARE SPECIFIED AS PART OF A DELEGATED DESIGN AND SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE PROJECT STATE.
- SHOP DRAWINGS AND CALCULATIONS SHALL BE PROVIDED FOR REVIEW AND APPROVAL BY THE SEOR AND THE ARCHITECT.
 - STEEL STAIRS/RAILING/GUARDRAILS

SHEATHING

- ROOF SHEATHING SHALL BE 5/8" NOMINAL THICK APA RATED SHEATHING WITH A MINIMUM SPAN RATING OF 32/16. ATTACHMENT SHALL BE PER MINIMUM APA STANDARDS FOR THE GIVEN EXPOSURE AND WIND SPEED OR AS INDICATED IN THESE DRAWINGS, WHICHEVER IS MORE STRINGENT.
- WALL SHEATHING SHALL BE 1/2" X 4" INCH WOOD STRUCTURAL PANELS. ATTACHMENT SHALL BE PER MINIMUM APA STANDARDS FOR THE GIVEN EXPOSURE AND WIND SPEED OR AS INDICATED IN THESE DRAWINGS, WHICHEVER IS MORE STRINGENT. NOTE: NAILING PATTERNS AT SHEAR WALLS. THE STRUCTURAL SHEATHING SHALL BE FOR THE FULL WALL HEIGHT, AND WHERE OPENINGS OCCUR, THE WALL SHALL BE EXTENSIVELY SHEATHED INCLUDING AREAS ABOVE AND BELOW THE OPENINGS. FOR TWO OR MORE STORES, SHEATHING SHALL BE CONTINUOUS FOR 14'-0" ABOVE AND BELOW THE FLOOR PLATE.
- FLOOR SHEATHING SHALL BE 3/4" NOMINAL THICK APA RATED SHEATHING WITH A MINIMUM SPAN RATING OF 48/24. ATTACHMENT SHALL BE PER MINIMUM APA STANDARDS OR AS INDICATED IN THE DRAWINGS, WHICHEVER IS MORE STRINGENT, WHERE PROJECT SPECIFICATIONS OR JOIST MANUFACTURER DIRECT, AN FLOOR SHEATHING TO JOIST ADHESIVE SHALL BE USED.

COLD-FORM METAL FRAMING

- CMF SHALL BE DESIGNED ACCORDING TO THE AMERICAN IRON AND STEEL INSTITUTE (AISI) S100 (LATEST EDITION).
- ALL STRUCTURAL LOAD BEARING MEMBERS SHALL BE FORMED FROM CORROSION RESISTANT STEEL CORRESPONDING TO THE REQUIREMENTS OF ASTM A446 WITH A MINIMUM YIELD STRENGTH (Fy) = 40 KSI FOR STUDS, GRADE A, 33 KSI FOR T TRACK. ALL STRUCTURAL MEMBERS SHALL BE ZINC COATED MEETING REQUIREMENTS FOR ASTM A525.
- MEMBERS SHALL BE INSTALLED LEVEL AND TRUE IN A WORKMANLIKE MANNER, INSTALL STRAPPING AND ACCESSORIES AS DETAILED AND AS REQUIRED FOR PROPER INSTALLATION.
- THE PHYSICAL AND STRUCTURAL PROPERTIES LISTED BY THE MANUFACTURER SHALL BE CONSIDERED THE MINIMUM PERMITTED FOR ALL FRAMING MEMBERS.

PRE-ENGINEERED CFS ROOF TRUSSES

- UNLESS OTHERWISE NOTED, THE PROJECT SHALL FOLLOW AISC CODE OF STANDARD PRACTICE FOR COLD-FORMED STRUCTURAL FRAMING, LATEST EDITION.
- ALL PREFABRICATED METAL TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN STRICT ACCORDANCE WITH THE APPLICABLE CODES AND SPECIFICATIONS TO SUPPORT ALL LIVE LOADS, DEAD LOADS, AND CONCENTRATED LOADS. LATERAL BRACING (DIAGONAL AND LATERAL BRIDGING), BOTH TEMPORARY AND PERMANENT, SHALL BE DESIGNED, PROVIDED AND NOTED ON ERECTION DRAWINGS BY THE MANUFACTURER. TEMPORARY BRACING SHALL REMAIN UNTIL PERMANENT BRACING AND THE ROOF DECK ARE COMPLETELY INSTALLED.
- PROVIDE EAVE BRACING DETAILS, ETC. AS REQUIRED TO INSURE PLUMB, LEVEL, STRUCTURAL BASE FOR EAVE TRIM AND CORNICE. NO TWISTING OR WARPING OF TRUSS ENDS WILL BE ACCEPTED PRIOR TO INSTALLATION OF CORNICE AND TRIM.
- ALL TRUSSES SHALL BE DESIGNED AND ANCHORED TO WITHSTAND THE NOTED WIND LOADS. THE ROOF TRUSSES SHALL BE DESIGNED AND ANCHORED FOR THE FOLLOWING LOADS:
 - TOP CHORD LIVE LOAD 20 PSF
 - TOP CHORD DEAD LOAD 10 PSF
 - BOTTOM CHORD LIVE LOAD 0 PSF
 - BOTTOM CHORD DEAD LOAD 20 PSF (WHERE NOTED ON PLANS)
 - MECHANICAL AREA 20 PSF
- VERIFY ALL DIMENSIONS AND DETAILS SHOWN, AND ANY OTHER NON-STRUCTURAL CODES OR STANDARDS, EVERY EFFORT HAS BEEN MADE TO ACCOMMODATE AND COORDINATE THESE ITEMS, BUT FINAL COORDINATION AND COMPLIANCE IS THE RESPONSIBILITY OF THE ARCHITECT, PROJECT MANAGER, OR OWNER AS IS APPLICABLE FOR THE PROJECT.
- SUBMIT DESIGN DRAWINGS AND CALCULATIONS BEARING THE REGISTERED PROFESSIONAL ENGINEERS SEAL OF THE DESIGN ENGINEER.

DESIGN LOADS AND PARAMETERS

- LIVE LOADS:
 - ROOF 20 PSF (REDUCIBLE)
 - TYPICAL FLOOR 40 PSF
 - CLASSROOMS 40 PSF
 - STAGES 100 PSF
 - LOBBIES 100 PSF
- DEAD LOADS
 - ROOF 10 PSF
 - ATTIC/CEILING 10 PSF
- SNOW LOAD 5 PSF
- WIND PARAMETERS:
 - DESIGN CODE ASCE 7-16
 - DESIGN WIND SPEED (ULT, 3 SEC GUST) 128 MPH
 - OCCUPANCY CATEGORY II
 - WIND EXPOSURE CATEGORY 4
 - INTERNAL PRESSURE COEFFICIENTS +-0.18
- SEISMIC PARAMETERS:
 - SEISMIC USE GROUP 2
 - SEISMIC IMPORTANCE FACTOR (I) 1.25
 - SITE CLASS D
 - SEISMIC DESIGN CATEGORY 5
 - MAPPED SPECTRAL RESPONSE ACCELERATION:
 - Ss 0.101
 - S1 0.066
 - SPECTRAL RESPONSE COEFFICIENTS:
 - S1/Ss 0.107
 - S2/Ss 0.106

APPLICABLE CODES

- UNLESS OTHERWISE NOTED OR SPECIFIED, ALL CONSTRUCTION SHALL CONFORM TO THE FOLLOWING CODES (LATEST EDITION UNLESS NOTED OTHERWISE):

IBC 2021	INTERNATIONAL BUILDING CODE
ASCE 7-16	MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES
ACI 318	AMERICAN CONCRETE INSTITUTE
ASCE 360	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISC 360	NORTH AMERICAN SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISC 302	CODE OF STANDARD PRACTICE FOR COLD-FORMED STEEL MEMBERS
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS (AS SPECIFIED IN CODES)
AWS D1.1	AMERICAN WELDING SOCIETY
SD-100	STANDARD FOR NON-CONCRETE STEEL FLOOR DECK
SD-400	STANDARD FOR STEEL ROOF DECK
SD-400	STANDARD SPECIFICATION OF STEEL FLOOR DECK
SJI 100	STANDARD FOR QUALITY CONTROL AND QUALITY ASSURANCE FOR INSTALLATION OF STEEL DECK
	POSSIBLE AGGREGATES, LAGERS, DRAG-AGES, AND JOIST ORDERS
TMS 402	BUILDING CODE FOR MASONRY STRUCTURES
TMS 602	SPECIFICATION FOR MASONRY STRUCTURES
W1998	DESIGN OF SLAB-ON-GRADE FOUNDATIONS WITH 1998 UPDATE

STANDARD ABBREVIATIONS AND SYMBOLS

A	CMU WALL WIDTH - LUNEL #	LLH	LONG LEG HORIZONTAL
Ø	AND	LLV	LONG LEG VERTICAL
Ø	DIAMETER	LVL	LAMINATED VENER LUMBER
A.F.F.	ABOVE FINISH FLOOR	MAX	MAXIMUM
ANCH	ANCHOR BOLT	MECH	MECHANICAL
ALT	ALTERNATE	MEP	MECHANICAL, ELECTRICAL, & PLUMBING
ARCH	ARCHITECTURAL / ARCHITECT	MIN	MINIMUM
B	BOTTOM OF	MISC	MISCELLANEOUS
B/L	BRICK LUNEL	NS	NEAR SIDE
BOT	BOTTOM	NTS	NOT TO SCALE
BPW	BASE PLATE W	O.C.	ON CENTER
BRNG	BRACING	Ø	OUTSIDE DIAMETER
BW	BOTH WAYS	O.S.F.	OUTSIDE FACE
C/C	CENTER TO CENTER	OPP.	OPPOSITE
C.J.	SAW CUT CONTROL JOINT	COND.	CONDITION SHOWN IN SECTION/DETAIL
CB	CONCRETE BEAM	P.E.J.	PREFORMED EXPANSION JOINT
CGH	CONCRETE COLUMN	P.T.	POST TENSION (CONCRETE)
CH	COLD FORM METAL FRAMING	PT	PRESSURE TREATED LUMBER
CP	CAST IN PLACE	PCF	POUNDS PER CUBIC FOOT
CL	CLEAR	CL	CLEAR
CL	CENTERLINE	PL	PLATE
CMU	CONCRETE MASONRY UNIT	PSF	POUNDS PER SQUARE FOOT
COL	COLUMN	PSI	POUNDS PER SQUARE INCH
COORD.	COORDINATE	RD	ROOF DRAIN
DEMO	DEMOLISH	REIN	REINFORCEMENT
DET.	DETAIL	REQD	REQUIRED
DL	DEAD LOAD	REV.	REMOVED
DWS	DRAWINGS	RO	ROUGH OPENING
E.J.	EXPANSION JOINT	SB	STUD PACKAGE OF STUDS
E.O.D.	EDGE OF DECK	SCH.	SCHEDULE
E.O.S.	EDGE OF SLAB	SEOR	STRUCTURAL ENGINEER OF RECORD
EA	EACH	SP	SPREAD FOOTING
EF	EACH FACE	SHT	SHEET
EL	ELEVATION	SIM.	SIMILAR TO INDICATED CONDITION W/ ONLY
EQ.	EQUAL	SOG	SLAB ON GRADE
EW	EACH WAY	SSL	SHORT SLOTTED HOLE
EXST.	EXISTING	EXT	EXTENSION
EXT	EXTENSION	STD.	STANDARD
F.C.J.	FORMED CONSTRUCTION JOINT	SWW	SHEAR WALL
F.F.E.	FINISH FLOOR ELEVATION OF STRUCTURE (IF PLYWOOD OR TOP OF CONCRETE)	SYM	SYMMETRICAL
F.D.	FLOOR DRAIN	TR	TRUSS
F.V.	FLOOR VENT	T.B.	TOP & BOTTOM
FEN	FOUNDATION	T.O.B.	TOP OF BEAM
FLR	FLOOR	T.O.F.	TOP OF FOOTING
FT	FEET	T.O.S.	TOP OF STEEL
FTE	FEET	T.O.W.	TOP OF WALL
FT	FEET	THRU	THROUGH
FTS	FEET	TRANS.	TRANSVERSE TYP.</td
GTW	GRID TRUSS	TRANS.	TRANSVERSE
H	HEADRAIL	UNO.	UNLESS NOTED OTHERWISE
HSS	HOLLOW STEEL SHAPE	VERT.	VERTICAL
ID.	INSIDE DIAMETER	W	WALL
ISF	INSIDE FACE	W.F.	WELDED WIRE FABRIC
IN.	INCH	W	WITH
K	KIP (1000 POUNDS)	WO	WITHOUT
LB	POUND	WB	WOOD BEAM
LL	LIVE LOAD	WF	WIDE FLANGE STEEL BEAM
		WT.	WEIGHT

A S0.1 TYP. BASE PLATE ELEVATION SCALE: 1 1/2" = 1'-0"

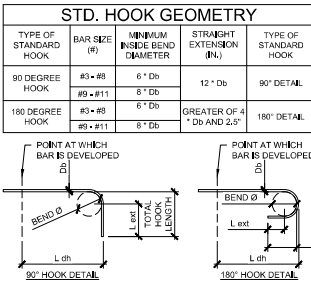
BP-1 S0.1 BASE PLATE DETAIL SCALE: 1 1/2" = 1'-0"

BP-2 S0.1 BASE PLATE DETAIL SCALE: 1 1/2" = 1'-0"

CONCRETE REINFORCEMENT LAP SPlice SCHEDULE					
CONCRETE COMPRESSIVE STRENGTH		3000 PSI		4000 PSI	
BAR SIZE (#)	SPlice TYPE	TOP BAR (IN.)	OTHER BAR (IN.)	TOP BAR (IN.)	OTHER BAR (IN.)
3	DEVELOPMENT LENGTH	21	16	18	14
	CLASS B LAP SPlice	28	21	24	18
	DEVELOPMENT LENGTH	28	22	25	19
4	DEVELOPMENT LENGTH	37	28	32	25
	CLASS B LAP SPlice	36	27	31	24
	DEVELOPMENT LENGTH	46	36	40	31
5	DEVELOPMENT LENGTH	43	33	37	28
	CLASS B LAP SPlice	42	32	36	27
	DEVELOPMENT LENGTH	54	42	48	37
6	DEVELOPMENT LENGTH	61	42	54	42
	CLASS B LAP SPlice	60	41	53	41
	DEVELOPMENT LENGTH	71	55	62	47
7	DEVELOPMENT LENGTH	69	51	60	47
	CLASS B LAP SPlice	68	50	59	46
	DEVELOPMENT LENGTH	80	62	72	55
8	DEVELOPMENT LENGTH	80	62	70	54
	CLASS B LAP SPlice	79	61	69	53
	DEVELOPMENT LENGTH	91	73	80	62
9	DEVELOPMENT LENGTH	80	62	70	54
	CLASS B LAP SPlice	79	61	69	53
	DEVELOPMENT LENGTH	91	73	80	62
10	DEVELOPMENT LENGTH	118	90	102	78
	CLASS B LAP SPlice	117	89	101	77
	DEVELOPMENT LENGTH	131	102	113	87
11	DEVELOPMENT LENGTH	131	102	113	87
	CLASS B LAP SPlice	130	101	112	86
	DEVELOPMENT LENGTH	144	113	125	98

REINFORCEMENT LAP SPlice NOTES

- "TOP BAR" INDICATES MORE THAN 12" OF FRESH CONCRETE BELOW SPlice (ℓ = 1.3).
- "OTHER BAR" INDICATES BAR WITH LESS THAN 12" OF FRESH CONCRETE PLACED BELOW SPlice (ℓ = 1.0).
- LAP SCHEDULE ASSUMES NORMAL WEIGHT CONCRETE (ℓ = 1.0).
- LAP SCHEDULE ASSUMES UNCOATED BARS (ℓ = 1.0).
- SPACING / CLEAR COVER REQUIREMENTS:
 1. CLEAR SPACING OF BARS BEING DEVELOPED OR LAP SPliced NOT LESS THAN ONE BAR DIAMETER. CLEAR COVER NOT LESS THAN ONE BAR DIAMETER AND STIRRUPS OR TIES THROUGHOUT LAP SPlice NOT LESS THAN CODE MINIMUM.
 2. CLEAR SPACING OF BARS BEING DEVELOPED OR LAP SPliced NOT LESS THAN 2 BAR DIAMETERS AND CONCRETE COVER NOT LESS THAN BAR DIAMETER.
 3. NOT AN ENGINEER OF RECORD IF CONDITIONS/ASSUMPTIONS ABOVE ARE NOT MET.



DEVELOPMENT LENGTH OF STD. 90° HOOK					
BAR SIZE (#)	FC	3000 PSI	4000 PSI	5000 PSI	
3		6	5	5	
		8	7	6	
		10	9	8	
4		12	10	9	
		14	12	11	
		15	14	12	
5		17	15	14	
		20	17	16	
		22	19	17	

REINFORCEMENT HOOK NOTES

- HOOK LENGTH ASSUMES NORMAL WEIGHT CONCRETE (ℓ = 1.0).
- HOOK LENGTH ASSUMES UNCOATED BARS (ℓ = 1.0).
- HOOK LENGTH ASSUMES SIDE COVER (NORMAL TO PLANE OF HOOK) ≥ 2ℓ AND FOR 90° HOOK WITH COVER ON BAR EXTENSION BEYOND HOOK ≥ 2ℓ (ℓ = 0.7).
- HOOK LENGTH ASSUMES NO CONFINEMENT REINFORCEMENT (ℓ = 1.0).

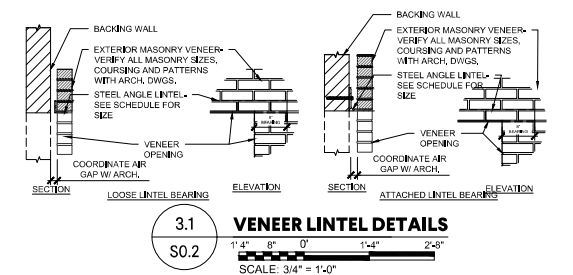
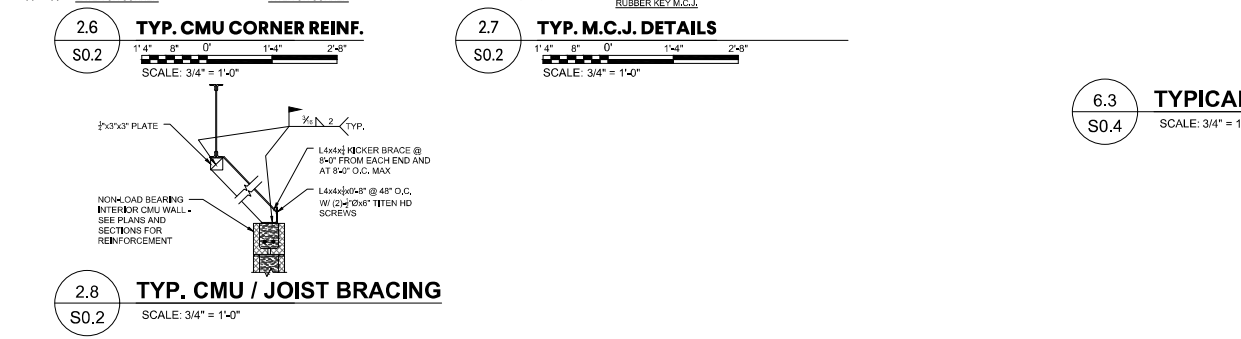
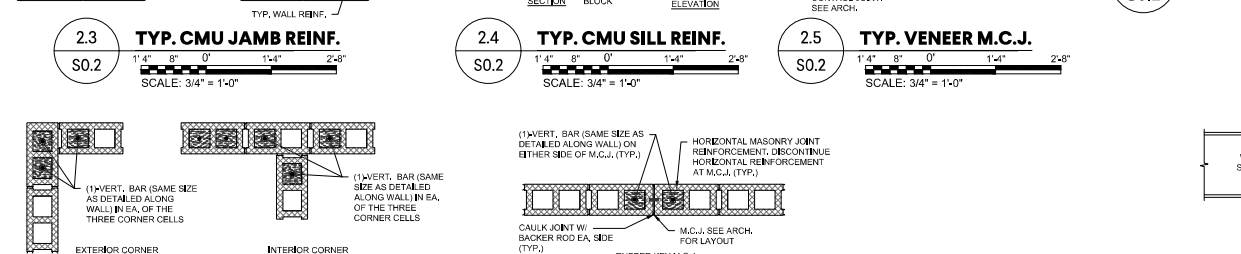
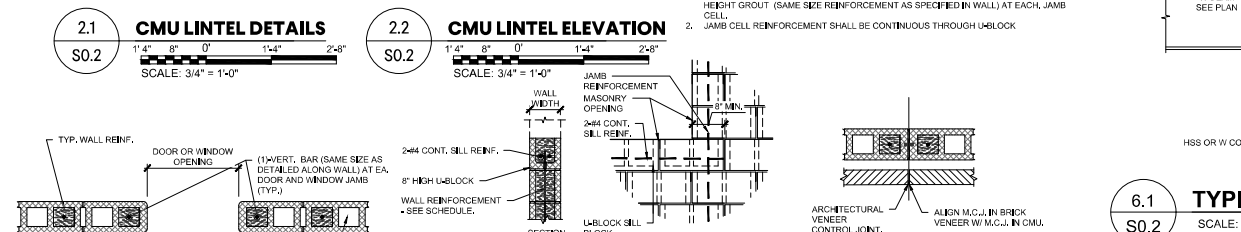
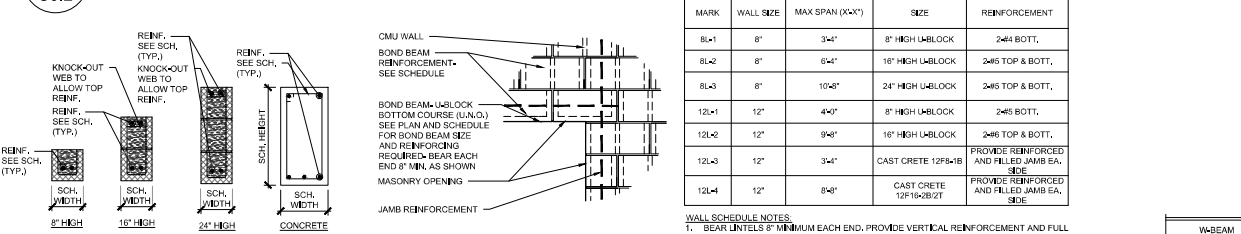
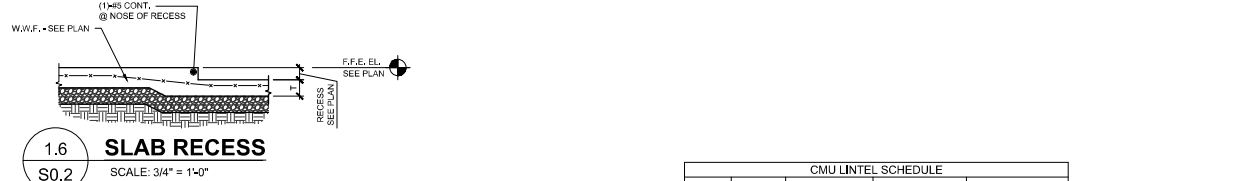
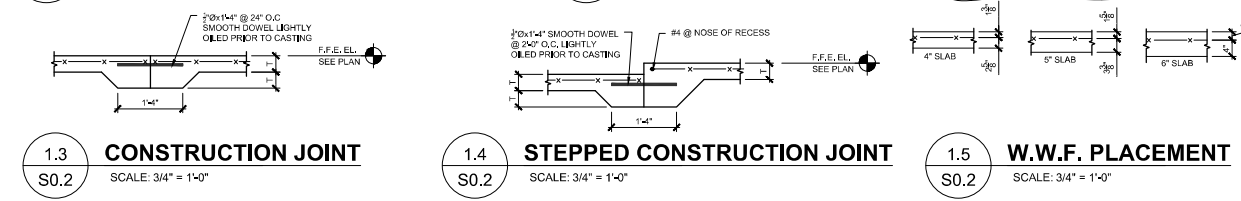
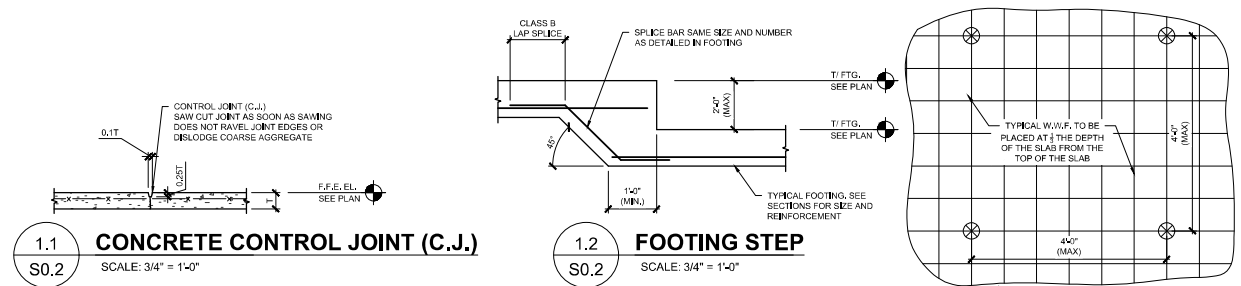
STD. 90° HOOK LENGTH		STD. 180° HOOK LENGTH	
BAR SIZE (#)	TOTAL HOOK LENGTH (IN.)	BAR SIZE (#)	TOTAL HOOK LENGTH (IN.)
3	6	3	4
4	8	4	4
5	10	5	5
6	12	6	6
7	14	7	7
8	15	8	7
9	16	9	9
10	20	10	10
11	22	11	11

CMU WALL REINFORCEMENT SCHEDULE					
MARK	LOCATION	SIZE	MAX HT.	VERT. REIN.	
W-1	INTERIOR	8" CMU	164"	#6 @ 48" O.C.	
W-2	EXTERIOR	12" CMU	264"	#6 @ 16" O.C.	

- WALL SCHEDULE NOTES:
* WALL HEIGHT MEASURED FROM FINISH FLOOR TO TOP OF BOND BEAM
** WALL HEIGHT MEASURED FROM OUTSIDE FINISH GRADE TO TOP OF WALL/GRADE

SPREAD FOOTING SCHEDULE			
MARK	SIZE	REIN. EA. WAY	
SP-2.5	2'-0" X 2'-0" X 1'-0"	2#6	
SP-4	1'-0" X 1'-0" X 1'-0"	4#6 TOP & BOTTOM	

- SPREAD FOOTING NOTES:
1. NOT ALL FOOTINGS ARE NECESSARILY USED
2. FOOTINGS

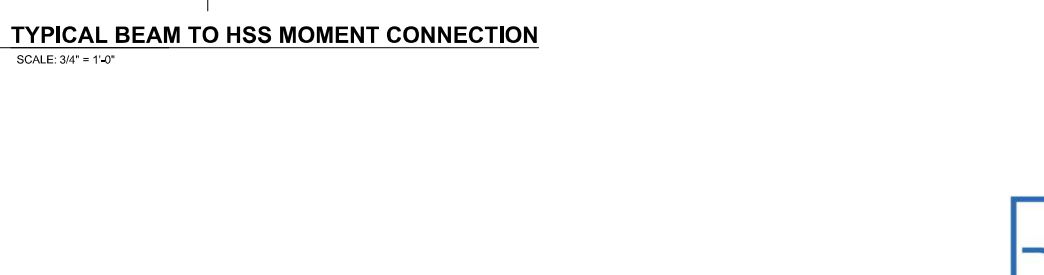
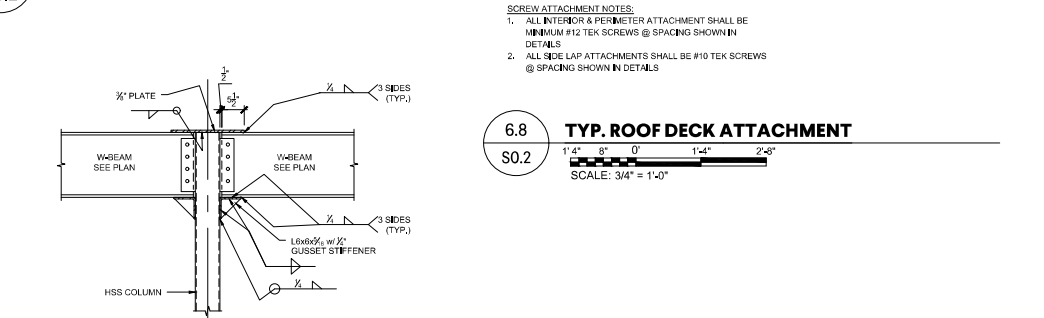
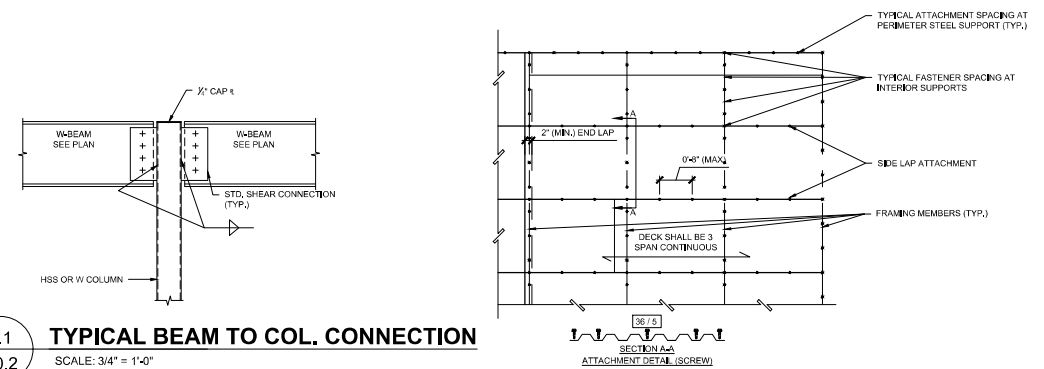


BRICK VENEER LINTEL SCHEDULE (BL-X)

MARK	WALL TYPE	MAX SPAN (X'-X")	SIZE	NOTES
BL-1	CMU	3'-4"	1-1/2" x 4-1/2"	
BL-2	CMU	6'-4"	1-1/2" x 6-1/2"	
BL-3	CMU	10'-4"	1-1/2" x 10-1/2"	
BL-4	CMU	>10'-4"	1-1/2" x 10-1/2" CONCRETE SCREW ANCHOR @ 8" O.C.	ATTACH TO BACKING WALL

BRICK VENEER SCHEDULE NOTES:

- ALL LINTELS SHALL BEAR 8" EA. SIDE U.N.O.
- ALL LINTEL SHALL BE LOOSE BRG. UNLESS SPECIFIED TO BE ATTACHED TO BACKING WALL.
- WHERE OPENING OCCUR IN WALLS LINTELS ARE NOT CALLED OUT. PROVIDE LINTEL FROM ABOVE SCHEDULE BASED UPON OPENING CLEAR SPAN.
- NOTIFY ENGINEER IF OPENING SPAN EXCEEDS SCHEDULE LENGTH.



Goshen High School Arts Center Renovation

To
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FOR THE
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TROY, ALABAMA

McKEE and ASSOCIATES
ARCHITECTURE and INTERIOR DESIGN

631 SOUTH HULL STREET MONTGOMERY, ALABAMA 36104 (205) 554-9553



SHEET TITLE : TYPICAL SECTIONS

McKEE JOB # : 20.119

DRAWN BY : TNS

DATE : 10.10.25

REVISED DATE :

REVISED DATE :

REVISED DATE :

SHEET NO. : S0.2



REQUIRED STRUCTURAL OBSERVATIONS FOR STRUCTURE IN ACCORDANCE WITH IBC 1704.6			
IBC REFERENCE	CONDITION REQUIRING STRUCTURAL OBSERVATION	DOES CONDITION EXIST ON THIS PROJECT (Y/N)	ADDITIONAL OBSERVATIONS REQUIRED
1704.6.1.1	RISK CAT. II OR IV	Y	SEE PROJECT DRAWINGS
1704.6.1.2	HIGH RISE BUILDING	N	
1704.6.1.3	SEISMIC DESIGN CATEGORY E & GREATER THAN 2 STORES	N	
1704.6.1.4	ADDITIONAL OBSERVATIONS REQUIRED BY SEQR	N	
1704.6.1.5	ADDITIONAL OBSERVATION REQUIRED BY BUILDING OFFICIAL	VERIFY WITH AUTHORITY HAVING JURISDICTION	VERIFY WITH AUTHORITY HAVING JURISDICTION

DESIGNATED SEISMIC / WIND RESISTANCE SYSTEM			
IBC REFERENCE	PROJECT CONDITION	DOES CONDITION EXIST (Y/N)	DESIGNATED WIND RESISTANCE SYSTEM IN ACCORDANCE IBC 1704.3.3
1705.12.1	WIND EXPOSURE B, WHERE V=150MPH OR GREATER	N	
1705.12.2	WIND EXPOSURE C OR D WHERE V=140 MPH OR GREATER	N	
IBC REFERENCE	PROJECT CONDITION	DOES CONDITION EXIST (Y/N)	DESIGNATED SEISMIC RESISTANCE SYSTEM IN ACCORDANCE IBC 1704.3.2
1705.13.1.1	STRUCTURAL STEEL, SFRS IN SEISMIC DESIGN CATEGORIES B, C, D, E, AND F	N	
1705.13.1.2	STRUCTURAL STEEL ELEMENTS FOR THE SFRS IN SEISMIC DESIGN CATEGORIES B, C, D, E, AND F	N	

IBC TABLE 1705.3 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION			
TYPE	CONTINUOUS	PERIODIC	REFERENCED STANDARD
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT		X	ACI 318: 20, 23.2, 25.3, 26.6, 106.6.3
2. REINFORCING BAR WELDING			AWS D1.4, ACI 318: 26.6.4
A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706		X	
B. INSPECT SINGLE PASS FILLET WELDS, MAXIMUM $\frac{1}{8}$ "		X	
C. INSPECT ALL OTHER WELDS	X		
3. INSPECT ANCHORS CAST IN CONCRETE		X	ACI 318: 17.6.2
4. INSPECT ANCHORS POST INSTALLED IN HARDENED CONCRETE MEMBERS			ACI 318: 17.6.2.4, 17.6.2
A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARD INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X		
B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A		X	
5. VERIFY USE OF REQUIRED DESIGN MIX		X	
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, DETERMINE THE TEMPERATURE OF THE CONCRETE	X		ASTM C31, ASTM C172, ACI 318: 26.6, 26.12
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X		ACI 318: 26.5
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X	ACI 318: 26.5.3, 26.5.5
9. INSPECT PRESTRESSED CONCRETE			ACI 318: 26.10
A. APPLICATION OF PRESTRESSING FORCES	X		
B. GROUTING OF BONDED PRESTRESSING TENDONS	X		
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS		X	
11. FOR PRECAST CONCRETE DIAPHRAGM CONNECTION OR REINFORCEMENT AT JOISTS CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS (MOE OR HES) IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C,D,E, OR F, INSPECT SUCH CONNECTIONS AND REINFORCEMENT IN THE FIELD			ACI 318: 26.13, 1.3 ACI 550.5
A. INSTALLATION OF THE EMBEDDED PARTS	X		
B. COMPLETION OF THE CONTINUITY OF REINFORCEMENT ACROSS JOINTS	X		
C. COMPLETION OF CONNECTIONS IN THE FIELD	X		
12. INSPECT INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTIONS FOR COMPLIANCE WITH ACI 550.5		X	ACI 318: 26.13, 1.3
13. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		X	ACI 318: 26.11.2
14. INSPECT FORM/WORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBERS BEING FORMED.		X	ACI 318 26.11.1, 2(b)

TABLE 1705.6 REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS		
TYPE	CONTINUOUS	PERIODIC
VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY		X
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL		X
PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS		
DURING FILL PLACEMENT, VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.		X
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT THE SITE HAS PREPARED PROPERLY.	X	
CONCEALED CONNECTIONS		X

SCHEDULE OF SPECIAL INSPECTIONS PER IBC					
IBC REFERENCE	MATERIAL / SYSTEMS / COMPONENTS / WORK	REQD (Y/N)	TYPE / EXTENT INSPECTION OR TEST REFERENCED STANDARD	PERIODIC / CONTINUOUS	ADDITIONAL REQUIREMENTS
SPECIAL CASES					
1705.1.1.1	MATERIAL & SYSTEMS ALTERNATIVES TO THAT PRESCRIBED BY CODE	N		P	
1705.1.1.2	UNUSUAL DESIGN APPLICATIONS	N		P	
1705.1.1.3	MATERIALS & SYSTEMS REQUIRED TO BE INSTALLED IN ACCORDANCE WITH ADDITIONAL MANUFACTURER'S INSTRUCTIONS	N		P	
STEEL CONSTRUCTION					
1705.2.1	STRUCTURAL STEEL	Y	AISC 360 REQUIREMENTS	SEE AISC CHAPTER N	
1705.2.2	COLD-FORMED STEEL DECK	N	SDI/AISC REQUIREMENTS	SEE SDI/AISC APPENDIX 1	
1705.2.3	OPEN WEB STEEL JOISTS AND JOIST ORDERS	N	SEE IBC TABLE 1705.2.3		
1705.3.4	COLD-FORMED TRUSSES SPANNING 60' OR GREATER	N	VERIFY ALL DETAILS IN ACCORDANCE W/ APPROVED TRUSS DRAWINGS	P	
CONCRETE CONSTRUCTION					
1705.3.1	WELDING OF REINFORCING BARS	N	AWS D1.4 REQUIREMENTS	SEE SPEC	
1705.3.2	MATERIAL TEST	Y	ACI 318 CH. 19 & 20 REQUIREMENTS	SEE SPEC	
MASONRY CONSTRUCTION					
1705.4.1	GLASS UNIT MASONRY AND MASONRY VENEER IN RISK CATEGORY IV	N	TMS 602 LEVEL 2	SEE TMS 602 TABLE 4	
1705.4.2	VERTICAL MASONRY FOUNDATION ELEMENTS	Y	TMS 602 LEVEL 2	SEE TMS 602 TABLE 4	
WOOD CONSTRUCTION					
1705.5.1	HIGH LOAD DIAPHRAGMS	N	VERIFY ALL CONSTRUCTION IN ACCORDANCE WITH CONSTRUCTION DOCUMENTS	P	
1705.5.2	METAL PLATE CONNECTED WOOD TRUSSES SPANNING 60' OR GREATER	N	VERIFY ALL DETAILS IN ACCORDANCE W/ APPROVED TRUSS DRAWINGS	P	
1705.5.3	MASS TIMBER CONSTRUCTION (TYPE I, II, A-B, AND D+C CONSTRUCTION)	N		P	
SOILS					
1705.6	SPECIAL INSPECTION AND TEST OF EXISTING SITE SOIL CONDITIONS	Y		P	
FOUNDATIONS					
1705.7	SPECIAL INSPECTION AND TEST OF DURING INSTALLATION OF DRIVEN DEEP FOUNDATION ELEMENTS	N		C	
1705.8	SPECIAL INSPECTION AND TEST OF DURING INSTALLATION OF CAST-IN-PLACE DEEP FOUNDATION ELEMENTS	N		C	
1705.9	EQUIPMENT USED, PILE DIMENSIONS, TIP ELEVATIONS, FINAL DEPTH, FINAL INSTALLATION TORQUE, & ANY OTHER REQUIRED DATA	N		C	
1705.10	WHEN THERE IS A REASONABLE DOUBT AS TO THE STRUCTURAL INTEGRITY OF A DEEP FOUNDATION ELEMENT, AN ENGINEERING ASSESSMENT SHALL BE REQUIRED	N		C	
FABRICATED ITEMS					
1705.11	SPECIAL INSPECTION OF FABRICATED ITEMS IN ACCORDANCE WITH IBC 1704.2.6	N		P	
SPECIAL INSPECTIONS FOR WIND RESISTANCE					
1705.12.1	STRUCTURAL WOOD	N		P	
	FIELD GLUING	N		C	
	NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF ELEMENTS IN THE MEMBR	Y		P	
1705.12.2	COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION	Y		P	
	WELDING	Y		P	
	SCREW ATTACHMENT, BOLTING, ANCHORING, OTHER FASTENING OF ELEMENTS IN THE MEMBR	Y		P	
1705.12.3.1	ROOF COVERING, ROOF DECKING, AND ROOF FRAMING CONNECTIONS	Y		P	
1705.12.3.1	EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGMS AND FRAMING	Y		P	
SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE					
1705.13.1	STRUCTURAL STEEL	N		P	
1705.13.1.1	SFRS IN SEISMIC DESIGN CATEGORIES B, C, D, E, AND F	N		P	NOTE EXCEPTIONS
1705.13.2	SFRS ASSIGNED TO DESIGN CATEGORIES C, D, E, AND F	N		P	
	FIELD GLUING	N		C	
	NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF ELEMENTS IN THE MSFRS	N		P	
1705.13.3	COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION	N		P	
	WELDING	N		P	
	SCREW ATTACHMENT, BOLTING, ANCHORING, OTHER FASTENING OF ELEMENTS IN THE MEMBR	N		P	
1705.13.4	DESIGNATED SEISMIC SYSTEMS	N		P	
1705.13.5	ARCHITECTURAL COMPONENTS	N		P	NOTE EXCEPTIONS
1705.13.6	PLUMBING, MECHANICAL, AND ELECTRICAL COMPONENTS	N		P	NOTE EXCEPTIONS
1705.13.7	STORAGE RACKS	N		P	
1705.13.8	SEISMIC ISOLATIONS SYSTEMS	N		P	
1705.13.9	COLD-FORMED STEEL SPECIAL BOLTED MOMENT FRAMES	N		P	
TESTING FOR SEISMIC RESISTANCE					
1705.14.1	STRUCTURAL STEEL	N		P	
1705.14.1.1	SEISMIC FORCE-RESISTING SYSTEMS	N		P	NOTE EXCEPTIONS
1705.14.1.2	STRUCTURAL STEEL ELEMENTS	N		P	
1705.14.2	NONSTRUCTURAL COMPONENTS	N		P	
1705.14.3	DESIGNATED SEISMIC SYSTEMS	N		P	
1705.14.4	SEISMIC ISOLATION SYSTEMS	N		P	

CONCRETE INSPECTIONS AND TEST SHALL NOT BE REQUIRED FOR:
1. ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDING THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK
2. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDING THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE:
2.1. THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION
2.2. THE FOOTINGS ARE DESIGNED IN ACCORDANCE WITH TABLE 1809.7
2.3. THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH (f_c) NOT MORE THAN 2500 PSI, REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS
3. NONSTRUCTURAL CONCRETE SLABS SUPPORTED DIRECTLY ON THE GROUND, INCLUDING PRESTRESSED SLABS ON GRADE, WHERE THE EFFECTIVE PRESTRESS IN THE CONCRETE IS LESS THAN 150 PSI
4. CONCRETE FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.2
5. CONCRETE PATIOS, DRIVEWAYS AND SIDEWALKS ON GRADE
MASONRY SPECIAL INSPECTIONS AND TESTS SHALL NOT BE REQUIRED FOR:
1. EMPIRICALLY DESIGNED MASONRY, GLASS UNIT OR MASONRY VENEER DESIGNED IN ACCORDANCE WITH SECTIONS 2109, SECTION 2110, OR CHAPTER 14, RESPECTIVELY, WHERE THEY ARE PART OF THE A STRUCTURE CLASSIFIED AS RISK CATEGORY I, II, OR III.
2. MASONRY FOUNDATION WALLS CONSTRUCTED IN ACCORDANCE WITH TABLE 1807.1.6.3(1), 1807.1.6.3(1), 1807.1.6.3(2), 1807.1.6.3(3), OR 1807.1.6.3(4).
3. MASONRY FIREPLACES, MASONRY HEATERS OR MASONRY CHIMNEYS INSTALLED OR CONSTRUCTED IN ACCORDANCE WITH SECTION 2111, 2112, OR 2113, RESPECTIVELY



SHEET TITLE : SPECIAL INSPECTION SCHEDULE

MCKEE JOB # : 20.119

DRAWN BY : TNS

DATE: 10.10.25

REVISED DATE:

REVISED DATE:

REVISED DATE:

SHEET NO. : S0.3

Goshen High School Arts Center Renovation

To

Goshen High School

FOR THE

Pike County Board of Education

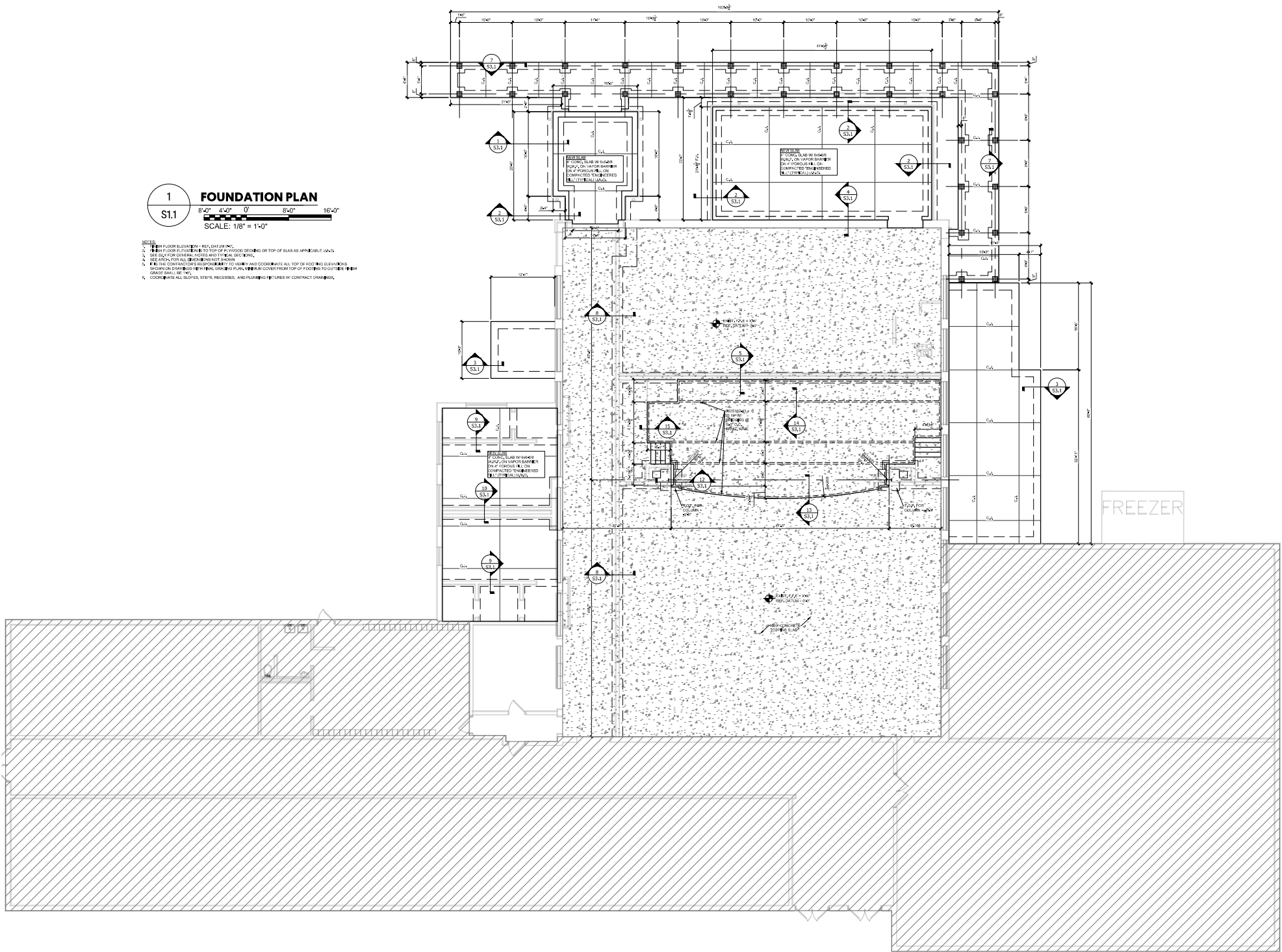
Troy, ALABAMA

McKEE and ASSOCIATES

ARCHITECTURE and INTERIOR DESIGN

631 SOUTH HULL STREET MONTGOMERY, ALABAMA 36104 (205) 554-9553





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TO
Goshen High School

FOR THE
Pike County Board of Education

TROY, ALABAMA

McKEE and ASSOCIATES
ARCHITECTURE and INTERIOR DESIGN
631 SOUTH HULL STREET MONTGOMERY, ALABAMA 36104 (205) 234-9553



SHEET TITLE : RENOVATION
FOUNDATION PLAN

McKEE JOB # : 20.119

DRAWN BY : TNS

DATE: 10.10.25

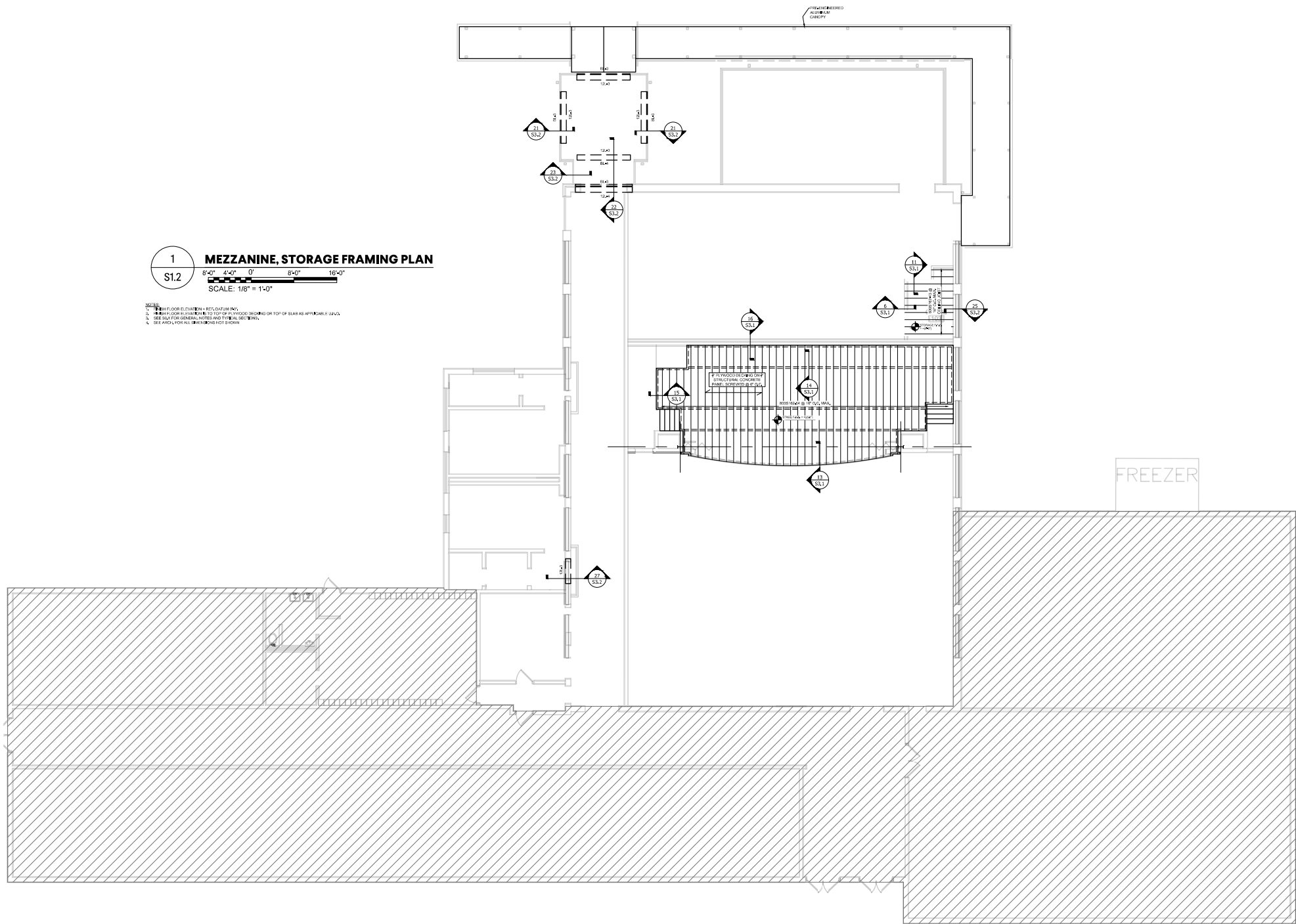
REVISED DATE:

REVISED DATE:

REVISED DATE:

SHEET NO. : **S1.1**





1
S1.2
MEZZANINE, STORAGE FRAMING PLAN
8'-0" 4'-0" 0' 8'-0" 16'-0"
SCALE: 1/8" = 1'-0"

NOTES:
1. ALL DIMENSIONS ARE TO CENTER UNLESS NOTED OTHERWISE.
2. ALL DIMENSIONS ARE TO TOP OF FINISHED FLOOR OR TOP OF SLAB AS APPLICABLE UNLESS NOTED OTHERWISE.
3. SEE PLAN FOR GENERAL NOTES AND TYPICAL SECTIONS.
4. SEE AREA FOR ALL DIMENSIONS NOT SHOWN.



SHEET TITLE : MID-LEVEL ENTRANCE, MEZZANINE AND STORAGE FRAMING PLAN
MCKEE JOB # : 20.119
DRAWN BY : TNS
DATE: 10.10.25
REVISED DATE:
REVISED DATE:
REVISED DATE:

SHEET NO. : S1.2

Goshen High School Arts Center Renovation

TO
Goshen High School

FOR THE
Pike County Board of Education

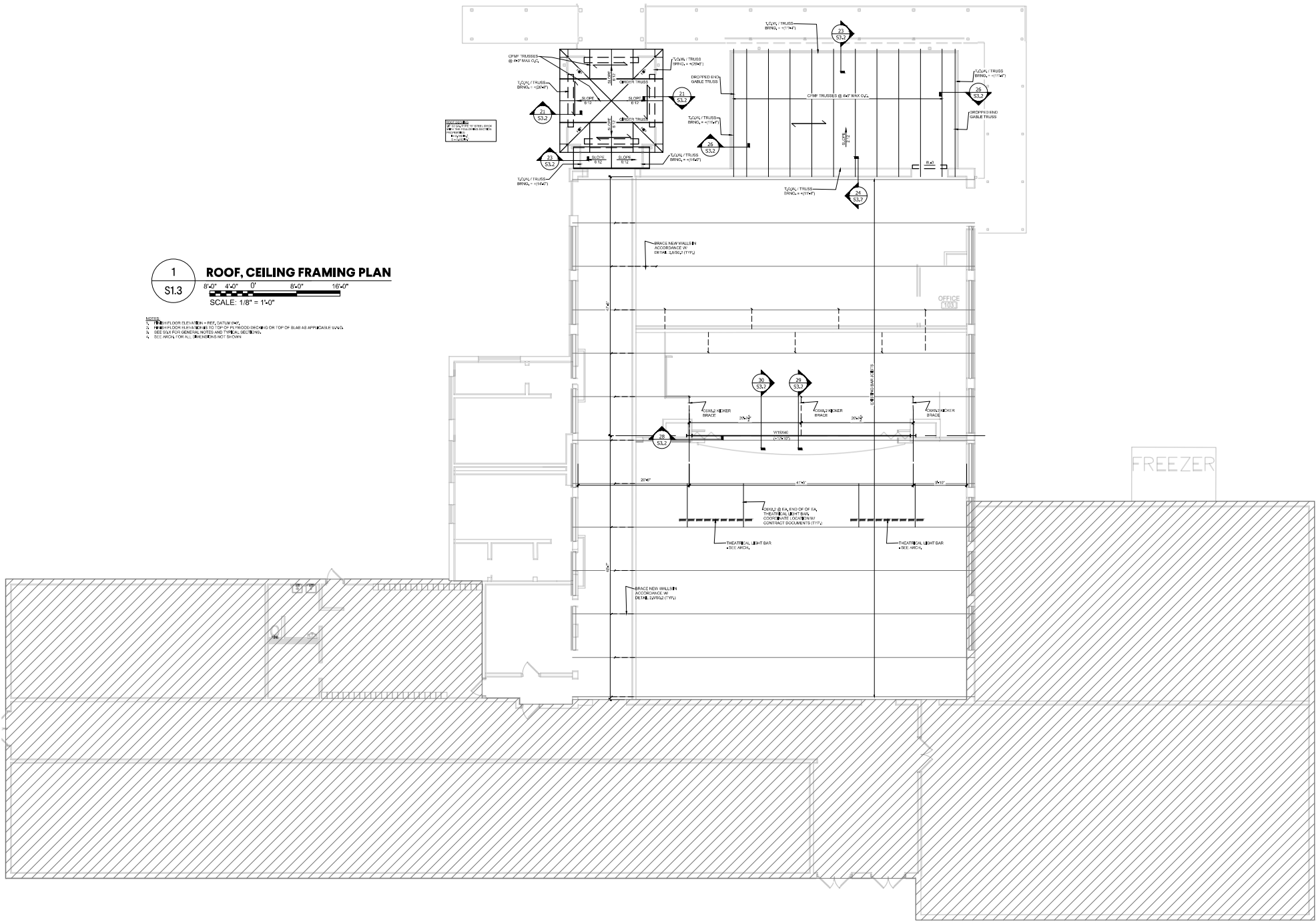
TROY, ALABAMA

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Goshen High School Arts Center Renovation

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Troy, ALABAMA

McKee and Associates

ARCHITECTURE and INTERIOR DESIGN

631 SOUTH HULL STREET MONTGOMERY, ALABAMA 36104 (205) 264-6653



SHEET TITLE : ENTRANCE ROOF AND RENOVATION CEILING FRAMING PLAN

McKee JOB # : 20.119

DRAWN BY : TNS

DATE : 10.10.25

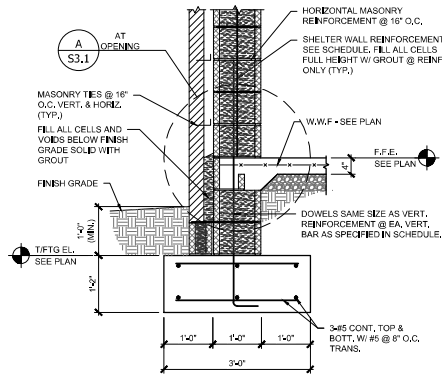
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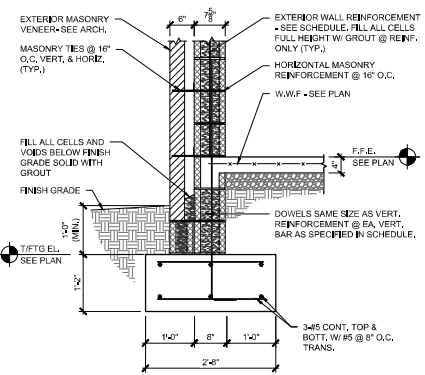
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SHEET NO. : S1.3

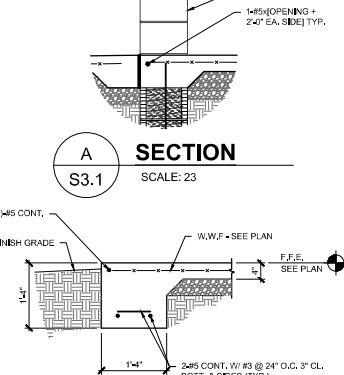




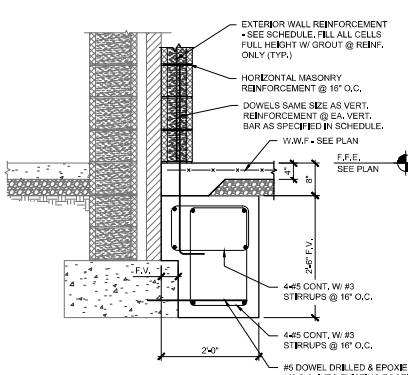
1 SECTION
S3.1 SCALE: 3/4" = 1'-0"



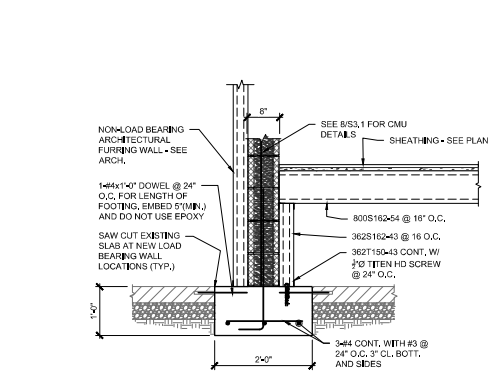
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S3.1 SCALE: 3/4" = 1'-0"



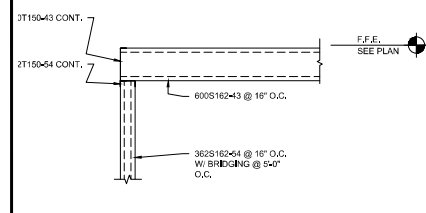
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S3.1 SCALE: 3/4" = 1'-0"



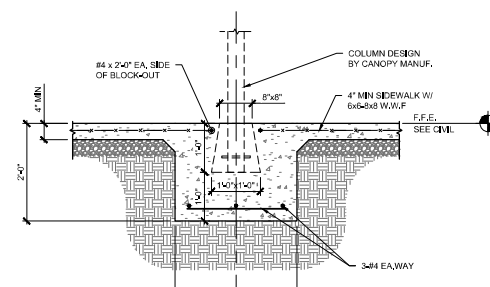
4 SECTION
S3.1 SCALE: 3/4" = 1'-0"



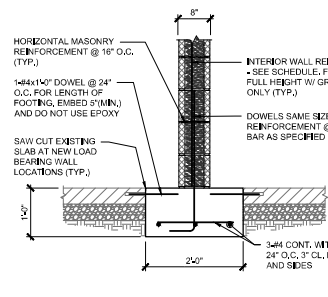
5 SECTION
S3.1 SCALE: 3/4" = 1'-0"



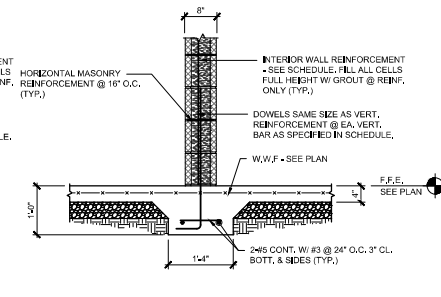
6 SECTION
S3.1 SCALE: 3/4" = 1'-0"



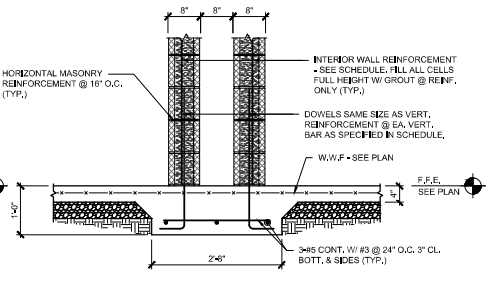
7 SECTION
S3.1 SCALE: 3/4" = 1'-0"



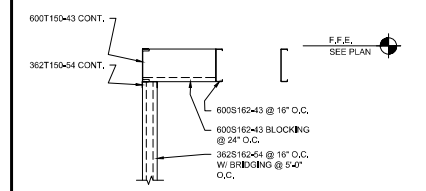
8 SECTION
S3.1 SCALE: 3/4" = 1'-0"



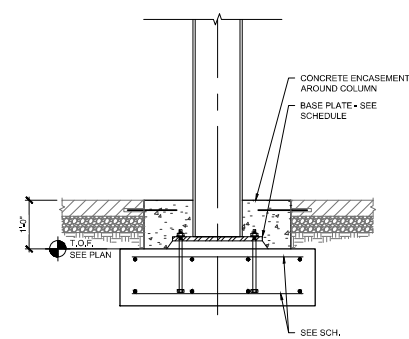
9 SECTION
S3.1 SCALE: 3/4" = 1'-0"



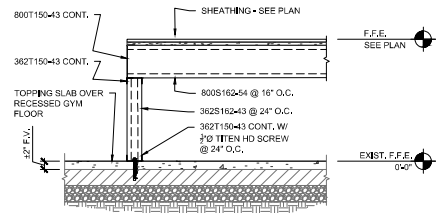
10 SECTION
S3.1 SCALE: 3/4" = 1'-0"



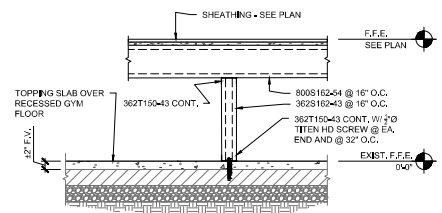
11 SECTION
S3.1 SCALE: 3/4" = 1'-0"



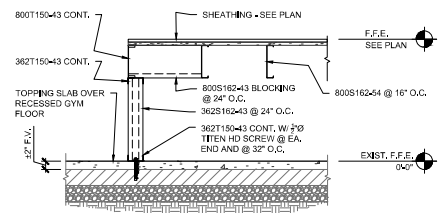
12 SECTION
S3.1 SCALE: 3/4" = 1'-0"



13 SECTION
S3.1 SCALE: 3/4" = 1'-0"



14 SECTION
S3.1 SCALE: 3/4" = 1'-0"



15 SECTION
S3.1 SCALE: 3/4" = 1'-0"

Goshen High School Arts Center Renovation

To Goshen High School

FOR THE Pike County Board of Education

Troy, ALABAMA

McKee and ASSOCIATES

ARCHITECTURE and INTERIOR DESIGN

631 SOUTH HULL STREET MONTGOMERY, ALABAMA 36104 (205) 264-9553



SHEET TITLE : SECTIONS

McKEE JOB # : 20.119

DRAWN BY : TNS

DATE : 10.10.25

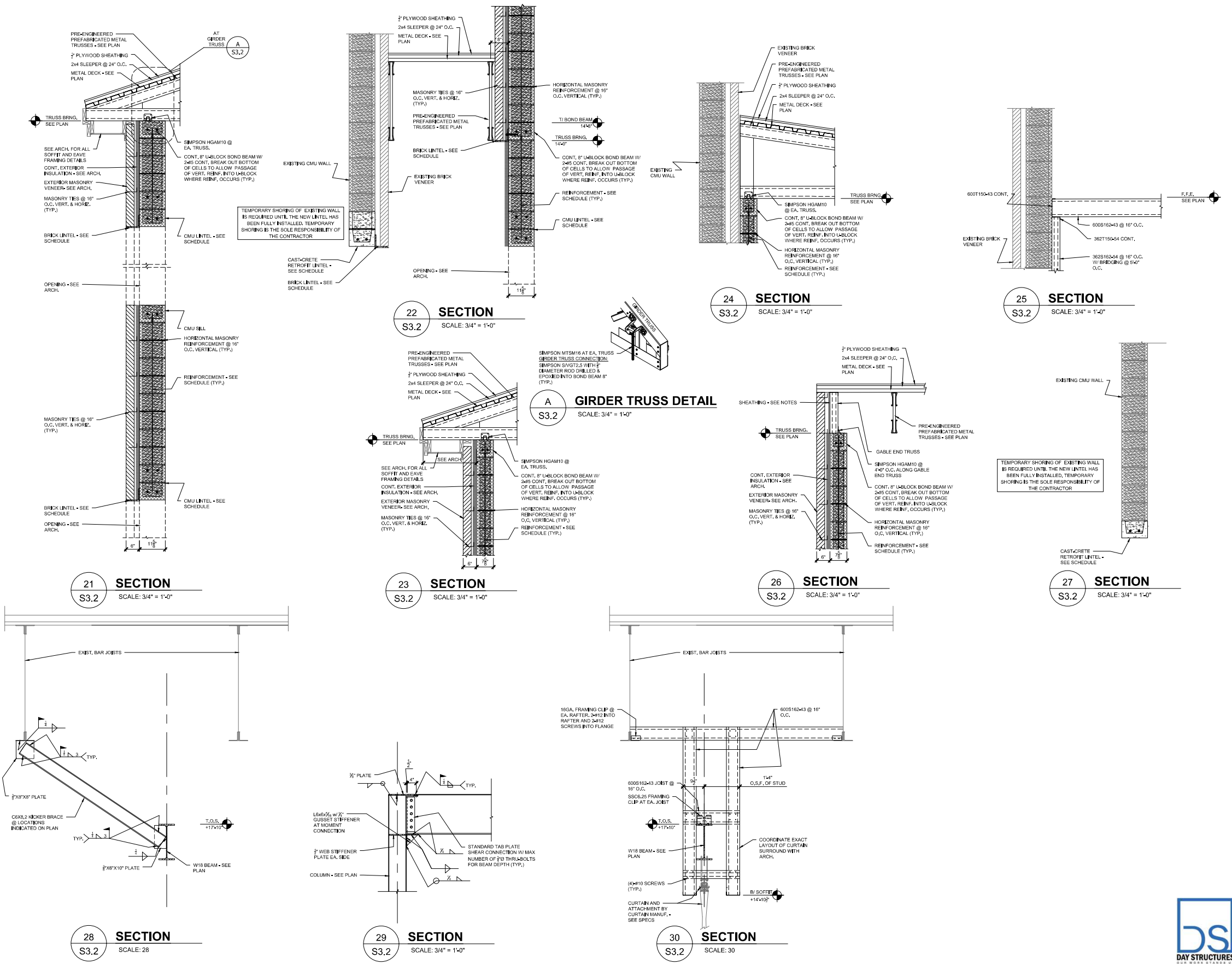
REVISED DATE :

REVISED DATE :

REVISED DATE :



SHEET NO. : S3.1



Goshen High School Arts Center Renovation

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SHEET TITLE : SECTIONS

McKee JOB # : 20.119

DRAWN BY : TNS

DATE : 10.10.25

REVISED DATE :

REVISED DATE :

REVISED DATE :



SHEET NO. : S3.2